



0 0.375 0.75 Miles

Proposed Collection System - Selma
Amberwood Project Updated 05/21/18--Nebraska/Highland Project Updated 12/14/21

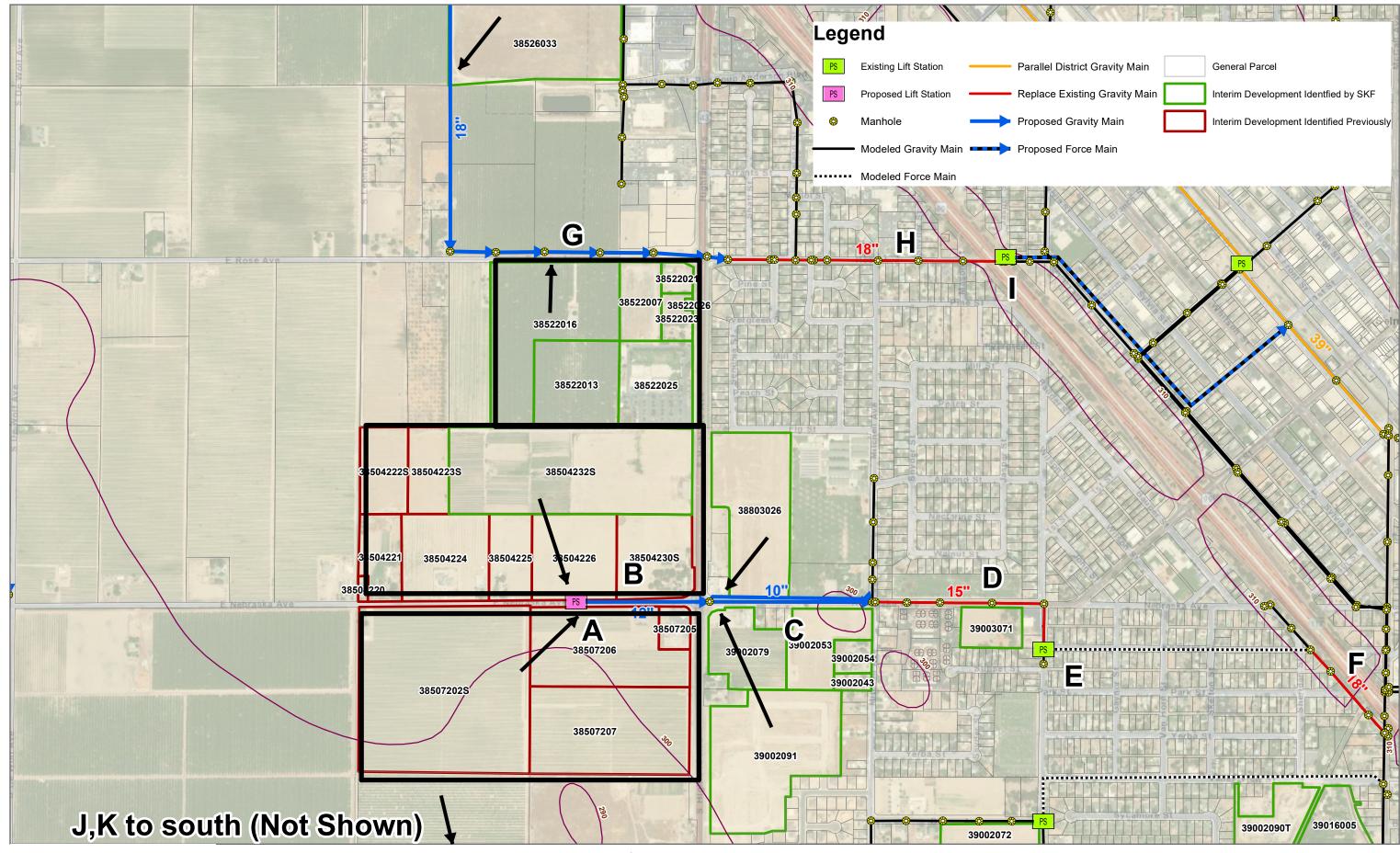




Table 1. Proposed Collection System - Selma Nebraska/Highland Development

| | | Estimated | Estimated | |
|----------------|---|---------------------|--------------------|--|
| | | Construction | Total Project | |
| Improvement | Description | Cost | Cost | Notes |
| | | | 4 | |
| Α | Proposed Lift Station serves area shown with a dead lift up to a gravity main. Design Capacity is 450 gpm. | \$724,800 | \$1,224,900 | |
| В | Proposed gravity main: 12-inch diameter. | \$224,600 | \$379,600 | |
| С | Parallel gravity main proposed alongside existing City 10-inch: 10-inch diameter. | \$246,500 | \$416,700 | |
| | This gravity main reach was previously identified as Project S-9 at 12-inch diameter. New diameter: 15-inch. | | | |
| | If instead the existing gravity main is to be revieved with a parallel gravity main, a 12-inch diameter parallel | | 4 | |
| D | main is required. | \$442,800 | \$748,300 | |
| | | | | Assumed that hydraulic and electrical components of lift station would require upgrade. |
| _ | Sunset Lift Station previously identified as sufficient. With intensified flows, requires capacity increase from | #750 400 | 4077 700 | Assumed that wet well would require reconfiguration and potential excavation. Assumed that |
| Е | 669 gpm to 900 gpm. | \$750,400 | \$975,500 | above ground structures were sufficient. |
| | Drawie velv identified as Drainet C. O with 40 inch dispertur. No shape required. Now vertical laws who were | | | Discussion with staff indicates that this will be a difficult against because of utility conflicts and |
| _ | Previously identified as Project S-9 with 18-inch diameter. No change required. New vertical layout/survey | \$413,100 | \$698,100 | Discussion with staff indicates that this will be a difficult project because of utility conflicts and potential permits/easements/coordination. |
| F | data needs to be updated to the model if layout is to change given conflicts and constraints in improvement. | \$413,100 | <i>\$096,100</i> | potential permits/easements/coordination. |
| | Previously identified as Project S-44 with new construction of 18-inch diameter gravity main. No change required. | \$690,100 | \$1,166,200 | |
| | • | | | |
| Н | Previously identified as Project S-22 with upsize to 18-inch diameter. No change required. | \$699,800 | \$1,182,600 | |
| | | | | Assumed that hydraulic and electrical components of lift station would require upgrade. |
| 1. | Rose Street Lift Station can accept flows from the tributary area on the figure without improvement. Build-out | ₾4 0 7 5 000 | £4 000 000 | Assumed that wet well will require reconfiguration due to increased flows. Assumed that above |
| I | design capacity required is increased to 1,900 gpm | \$1,075,600 | \$1,398,300 | ground structures were sufficient. |
| | Salma Crassing development (to the south not shown on figure) leaded as indicated, 214 perce are tributery | | | |
| | Selma Crossing development (to the south, not shown on figure) loaded as indicated. 214 acres are tributary to Clarkson Lift Station. To be conservative, all acreage was classified as commercial, with flow factor of 850 | | | |
| 1 . | gpd/acre. The resulting generation for Selma Crossing is 182,000 gpd of ADWF (126 gpm). | _ | _ | |
| J | Clarkson Lift Station (to the south, not shown on figure), has a current capacity of 1,500 gpm compared to a | <u> </u> | - | |
| | current design flow of 1,940 gpm. Thus, this lift station is deficient under current conditions. The addition of | | | |
| | the flows from the parcels shown on Figure 1 and Figure 2 increases the design flow to 3,400 gpm. The | | | Assumed that hydraulic and electrical components of lift station would require upgrade. |
| | buildout design flow for the lift station is currently 4,200 gpm. This compares to a predicted buildout design | | | Assumed that mydraulic and electrical components of first station would require upgrade. Assumed that wet well would require reconfiguration and potential excavation. Assumed that |
| l _K | flow of 3,000 gpm in the 2016 Master Plan. | \$1,731,240 | \$2,925,800 | above ground structures were sufficient. |
| 11 | 1 10 10 10 10 10 10 10 10 10 10 10 10 10 | ψ1,7 O1,2 TO | <i>\$2,020,000</i> | above greated at actained work cantionitie |

Notes: Estimated Construction Costs are conceptual-level costs appropriate to master planning. These costs assume that improvements are designed and constructed by the District. Estimated Total Costs include a 30% contingency in addition to 30% for District soft costs.